

April 27, 2009

Ms. Sharon Sears
Interactive Designs
308 West Erie Street, Suite 506
Chicago, Illinois 60610

RE: *Powell Replacement Elementary School, CPS, 7500 South Drive, Chicago, Illinois*
Dewatering Due Diligence Report by GeoSyntec Dated February 19, 2009

Dear Ms. Sears:

We have reviewed the GeoSyntec Report of February 19, 2009. We note that their comments relate to our draft report of June 23, 2008 which was superseded by our final report of July 3, 2008. Some of their comments had been addressed in our final report. Our response to their other comments are as follows:

1. Environmental assessment of the site was beyond the scope of our services. Phase one Environmental Assessment Study was performed by others.
2. It is not necessary to remove all the fill in the pavement areas or below the floor slab. In our opinion, 1.5 ft. of fill removal as recommended in our report is considered adequate. Proof rolling is to be done to detect the existence of hard spots from buried objects and old footings or to detect the zones of overly loose or soft soils. Only local excavation is needed to remove hard objects and loose soils, if encountered.
3. New fill and base coarse in paved areas should be as per IDOT gradation CA-6. In the floor slab area the recommended new fill is the same i.e., CA-6. The top 6 inches of the fill under the floor slab should be as per IDOT gradation CA-7 which is a clean fill and is intended to act as a capillary cut off layer and thus to minimize dampness in the floor slab.

4. Boring B-10 with deep fill is outside the building area. It is not anticipated that full removal of fill will be needed in the area outside the building and at the location of boring B-10. Boring number B-6 is in the building area. If footing excavation requires a deeper cut in this area, to remove the unsuitable fill a sloping cut can be made without encroaching on the public property.
5. The permeability rates given in the final report of July 3, 2008 agree with the rates given in our letter of July 3, 2008.

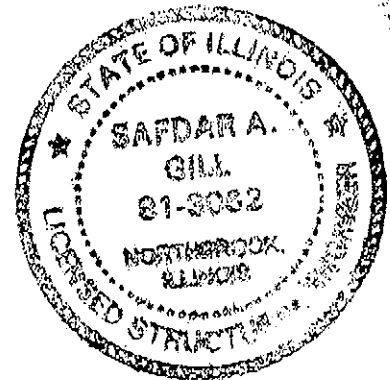
If there are any questions on this response please do not hesitate to contact us.

Very truly yours,

GROUND ENGINEERING CONSULTANTS, INC.

Safdar A. Gill

Safdar A. Gill, Ph. D., P.E.
Consultant



Exp. 11.30.10

FAX TRANSMISSION SHEET

TO: Ms Sharon Sears

COMPANY: Interactive Design

FAX NO. 312-482-9904

DATE: 2.26.09

FROM: Safdar A. Gill

Ground Engineering Consultants, Inc.

350 Pfingsten Road, Unit 106

Northbrook, Illinois, 60062-2032

FAX NO. 847-559-0181

PHONE NO. 847-559-0085

TOTAL NO. OF PAGES: 1

Re: Powell Replacement Elementary School:
Dewatering Due Diligence Report by GeoSynTec
dated 2.19.09.

Our final geotechnical report is dated July 3, 2008.
GeoSynTec comments relate to report of June 23, 2008
which is superseded by the July 3, 2008 Report.

The permeability rates given in the final report
agree with the rates given in our letter of July 3, 2008.

Compaction density of 95% is for the footing subgrade
& 90% for the floor slab subgrade.

Sincerely

Safdar A. Gill

April 30, 2009

Ms. Sharon Sears
Interactive Designs
308 West Erie Street, Suite 506
Chicago, Illinois 60610

RE: *Powell Replacement Elementary School, CPS, 7500 South Shore Drive, Chicago, Illinois*

Site Preparation

Dear Ms. Sears:

It is our understanding that the first phase of construction of the school will relate to site preparation work only. To this end the site preparation drawing has been prepared by Terra Engineering, Inc. which shows removal of unsuitable soils and backfilling with approved compacted fill for the construction of footings and the base slab. We were requested to clarify our recommendations contained in our Geotechnical Report relating to the extent of site preparation including the depth of excavation for the removal of unsuitable soils and backfilling. A summary of our recommendations is as follows:

1. In the floor slab area the existing fill and old footings should be removed to a depth of 1.5 ft. below the bottom of the floor slab.
2. In the footing area, if the existing footing top is within 1.5 to 3 ft. below the base of the new footing, the fill above the existing footing must be removed. The existing footing below that level can be left in place.
3. If the footings are deeper and the top of the existing footing is deeper than 3 ft. from the base of the new footing then the fill only needs to be removed to 3 ft. depth below the bottom of the proposed footing. The remaining fill and any footing which is deeper can be left in place. The 3 ft. of new fill will provide a bridging for the remaining fill and for support of the new footing.

4. The pool slab and foundations will not need to be removed provided they are a minimum of 3 ft. from the bottom of the proposed footings and the compaction requirements for the subgrade as outlined in our geotechnical report and as mentioned below can be met.

All new fill must be compacted to 95% of ASTM D-1557 maximum density for the footing area and to 90% of ASTM D-1557 maximum density in the floor slab area. Regarding the fill material to be placed in the excavated areas, non organic native sandy soils from the site are considered suitable. However, the available soils should be observed by a soil engineer and tested for Proctor density for controlling the fill density. We had recommended placing a 6 inch layer of clean drainage layer, such as CA-7 gradation, under the floor slab for minimizing dampness. It is desirable that this layer be placed at the time of floor slab construction otherwise it could get contaminated from other construction activities prior to slab construction.

If ground water is encountered during excavation for removal of deep footings or unsuitable fill soils, the over excavation can be backfilled with clean stone such as CA-1, in 9 to 12 inch layers within the water table and choked with CA-6. Once the fill is above the water level, it should be compacted in layers as described above.

It is our understanding that there may be some delay in the start of construction of the foundations following the site preparation work. It is likely that during this period the top portion of the fill may loosen from weathering, precipitation, freeze-for effects and from construction activities. We estimate that this disturbance may extend to a depth of about 6 inches from the finished grade during the site preparation work. When the general contractor starts the construction it will be necessary to re-compact the disturbed soil. The re-compaction must be checked by a Soil Engineer prior to placing the concrete.

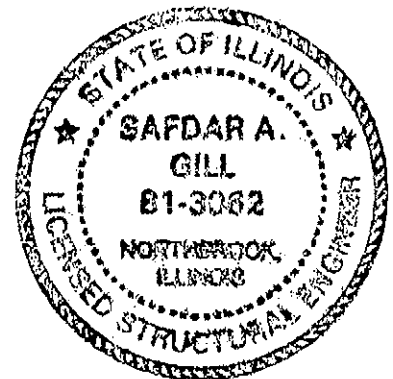
If there are any questions on the subject please do not hesitate to contact us.

Very truly yours,

GROUND ENGINEERING CONSULTANTS, INC.

Safdar A. Gill

Safdar A. Gill, Ph.D., P.E.
Consultant



Exp. 11-30-10

PARTITION TYPES

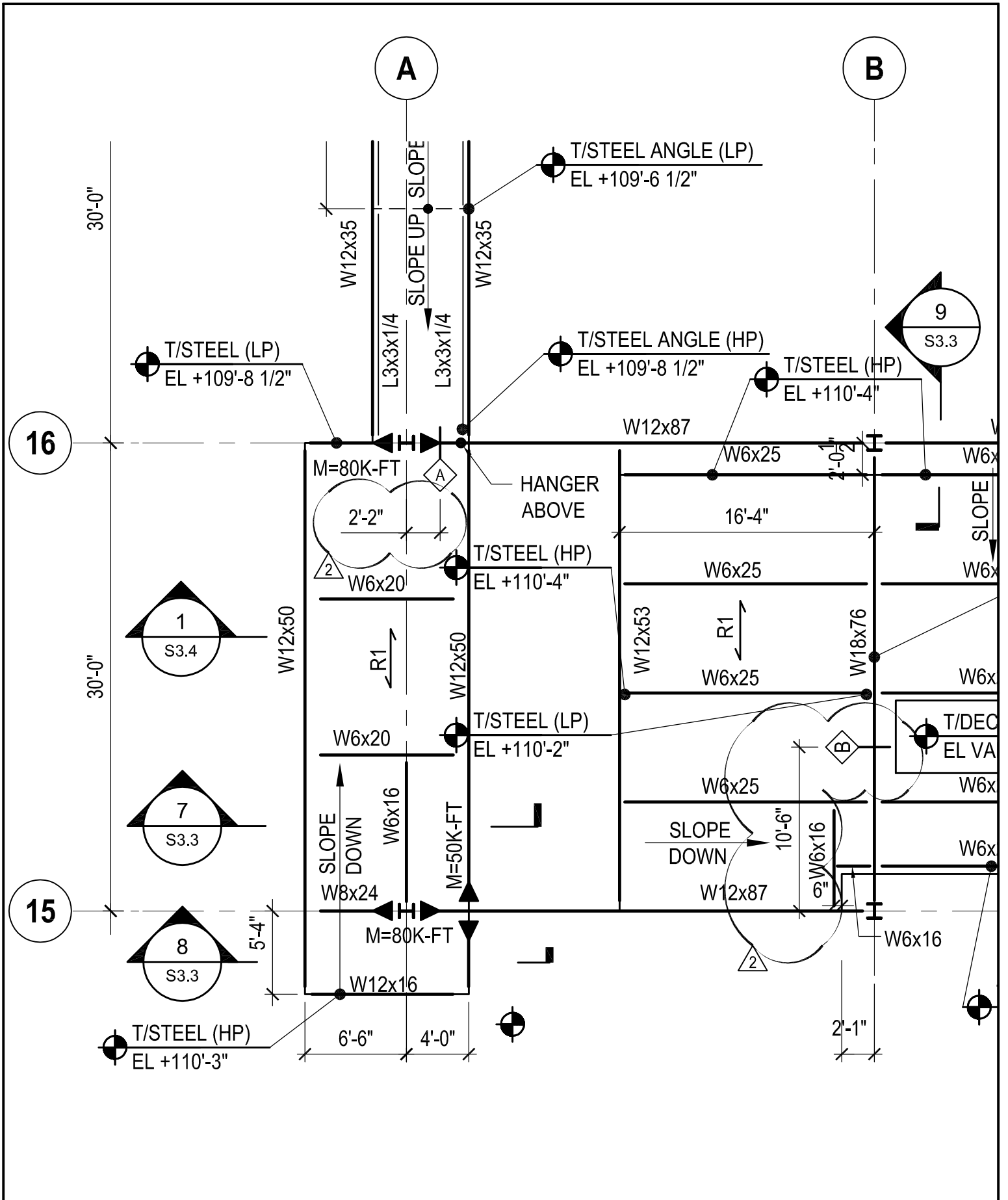
TYPE	LAYER DESCRIPTION	WIDTH A=ACTUAL P=PLAN	STUDS	INSULATION	RATING	U.L./TEST NUMBER
G2	(1) LAYER 5/8" GYPSUM BOARD EA SIDE (FULL HEIGHT)	A=4 7/8" P=4 7/8"	3 5/8" METAL C 25 GA	2" MINERAL FIBER BLANKET	1 HOUR STC-49	U465
G2A	(1) LAYER 5/8" ABUSE-RESISTANT GYPSUM BOARD EA SIDE (FULL HEIGHT)	A=4 7/8" P=4 7/8"	3 5/8" METAL C	2" MINERAL FIBER BLANKET	NONE	NONE
G3	(1) LAYER 5/8" GYPSUM BOARD ONE SIDE (FULL HEIGHT) & (2) LAYERS 5/8" GYPSUM BOARD OPPOSITE SIDE (FULL HEIGHT)	A=7 7/8" P=7 7/8"	6" METAL C	5 1/2" MINERAL FIBER BLANKET	NONE	NONE
G6C	(1) LAYER 5/8" ABUSE-RESISTANT GYPSUM BOARD EA SIDE (FULL HEIGHT)	SEE PLAN	3 5/8" METAL C	2" MINERAL FIBER BLANKET	NONE	NONE
G6CR	(1) LAYER 5/8" TYPE 'X' GYPSUM BOARD (FULL HEIGHT) EACH SIDE & (1) LAYER 5/8" TYPE 'X' ABUSE RESISTANT GYPSUM BOARD EACH SIDE (FULL HEIGHT)	SEE PLAN	6" METAL C 25 GA	5-1 1/2" MINERAL FIBER BLANKET	1+2 HOUR STC-50	U420
G12R	(2) LAYERS 5/8" TYPE 'X' GYPSUM BOARD EACH SIDE (FULL HEIGHT)	A=8 1/2" P=8 1/2"	6" METAL C 25 GA	5-1 1/2" MINERAL FIBER BLANKET	1+2 HOUR STC-55	U411
G13R	(1) LAYER 5/8" TYPE 'X' GYPSUM BOARD (FULL HEIGHT) EACH SIDE & (1) LAYER 5/8" TYPE 'X' ABUSE RESISTANT GYPSUM BOARD EACH SIDE (FULL HEIGHT)	A=8 1/2" P=8 1/2"	6" METAL C 25 GA	5-1 1/2" MINERAL FIBER BLANKET	1+2 HOUR STC-50	U411



**POWELL REPLACEMENT
ELEMENTARY SCHOOL**
7511 S. SHORE DRIVE
CHICAGO, IL 60649



InterActive Design **DATE:** 05/01/09
308 W. Erie Street, Suite 506 **SHEET REFERENCE:** A9.1
Chicago, Illinois 60654 **SKETCH NUMBER:** ASK-040
P: 312-482-8866 F: 312-482-9904



**POWELL REPLACEMENT
ELEMENTARY SCHOOL**

7511 S. SHORE DRIVE
CHICAGO, IL 60649



Thornton Tomasetti

Thornton Tomasetti, Inc.
330 N. Wabash Avenue, Suite 1500
Chicago, IL 60611
T 312.596.2000 F 312.596.2001

DATE: 05/01/09

SHEET REFERENCE: S1.2A

SKETCH NUMBER: **SSK-001**

BEAM WEB PENETRATION SCHEDULE

MARK	TYPE	MAX SIZE (IN)				PLATES (t x WIDTH (IN))				WELD	REM
		W	H	DIA	e	TOP NS & FS	BOTTOM NS & FS	SIDE NS & FS	A		
A	U	12	4	-	d/2	-	-	-	-	-	-
B	U	12	6	-	d/2	-	-	-	-	-	-

NOTES:

1. TYPE REFERS TO TYPICAL BEAM WEB PENETRATION DETAILS.
2. THE MINIMUM CLEAR SPACE BETWEEN TWO ADJACENT PENETRATIONS SHALL BE GREATER OF THE LARGEST PENETRATION DIMENSION OR TWO TIMES THE BEAM DEPTH UON.
3. SEE STRUCTURAL DRAWINGS FOR QUANTITY AND LOCATIONS OF PENETRATIONS. ALL BEAM PENETRATIONS SHALL BE SHOWN ON THE SHOP DRAWINGS FOR REVIEW BY THE ARCHITECT/ENGINEER. NO PENETRATIONS SHALL BE MADE WITHOUT PRIOR REVIEW OF THE ARCHITECT/ENGINEER.

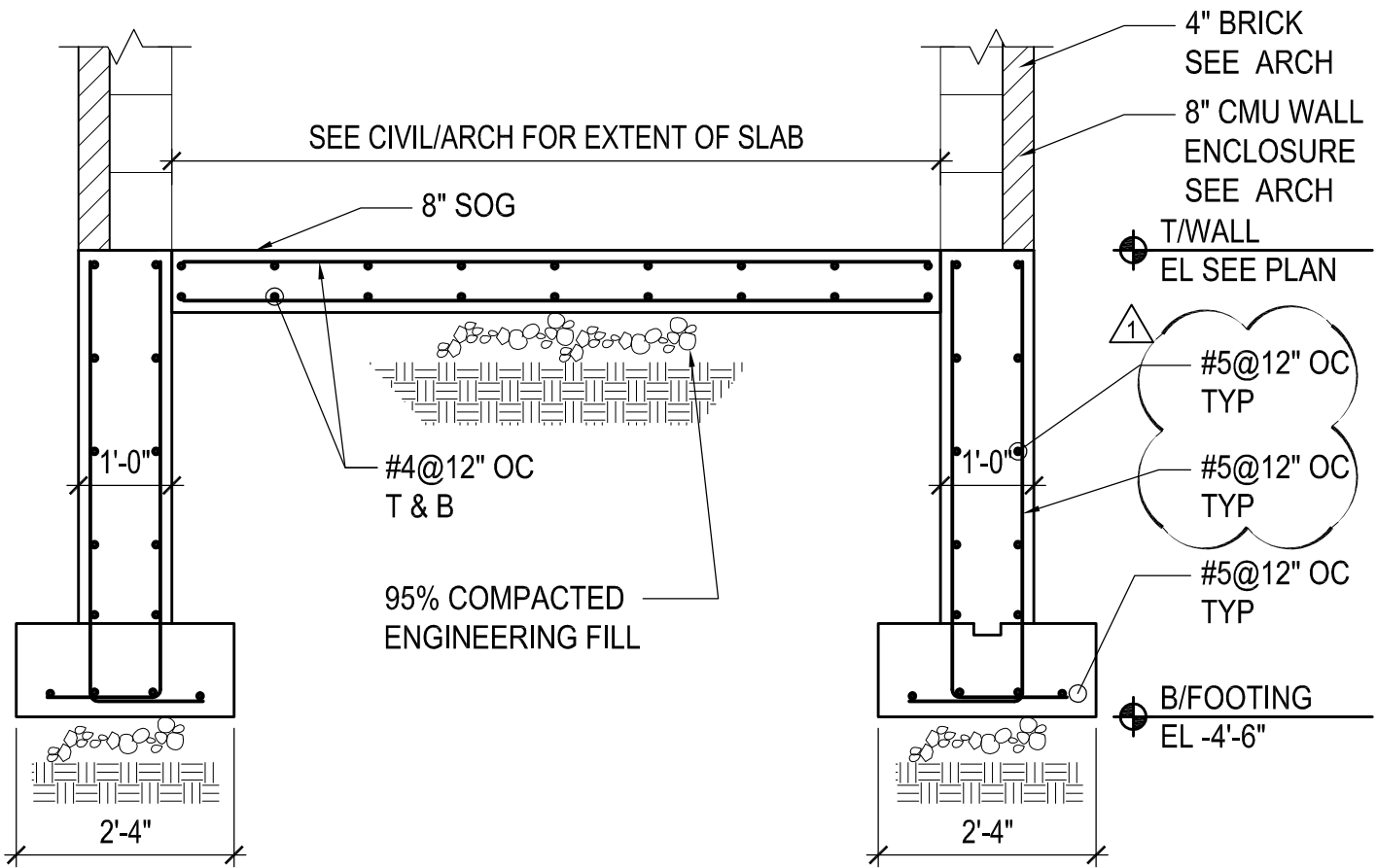


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DATE: 05/01/09
SHEET REFERENCE: S3.1
SKETCH NUMBER: **SSK-002**



10 SECTION AT TRANSFORMER ENCLOSURE
NOT TO SCALE



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DATE: 05/01/09
SHEET REFERENCE: S2.1
SKETCH NUMBER: **SSK-003**